King's Somborne

Hampshire

Drainage Note

November 2021



Project Information	n
Project:	King's Somborne
Report Title:	Drainage Note
Client:	King's Somborne Parish Council
Instruction:	The instruction to undertake this Drainage Note was received from Liz Manship of King's Somborne Parish Council
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	Document Histo	pry
Revision	Date	Comment
01	04/11/2021	First issue

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This report will remain valid for a period of twelve months (from the date of last issue) after which the source data should be reviewed in order to reassess the findings and conclusions on the basis of latest available information.









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Introduction

Waterco have been commissioned by King's Somborne Parish Council to undertake a Drainage Note in relation to potential future site allocations for residential development in King's Somborne.

The purpose of this Drainage Note is to establish the viability of draining surface water generated by up to 5no. development sites located throughout King's Somborne. Focus is given to:

- Estimating existing greenfield runoff rates.
- Establishing proposed surface water discharge rates (where off-site discharge to watercourse is required).
- Establishing whether gravity connection to Somborne Stream is achievable from proposed development surface water drainage systems (taking into account site levels, the invert level of below ground attenuation storage features, and outfall levels at Somborne Stream).
- Providing a typical detail of a below ground geo-cellular attenuation tank.
- Advising on any other drainage constraints and requirements for future developers to consider.

This Drainage Note should be read in conjunction with the Waterco Hydraulic Modelling Report (document reference 14083-HMN-01 – dated November 2021). The Hydraulic Modelling Report sets out the impact of surface water discharges from up to 5no. development sites to Somborne Stream on flood risk elsewhere throughout King's Somborne.

The 5no. development sites considered are:

- KS3 Developable area of 3,300m² Up to 15 dwellings
- KS5 (SHELAA 148b) Developable area of 3,100m² Up to 15 dwellings
- KS7 (SHELAA 80) Developable area of 1,600m² Up to 7 dwellings
- SHELAA 81 Developable area of 1,600m² Up to 7 dwellings
- KS6 Developable area of 900m² Up to 4 dwellings.

A location plan and an aerial image are included in Appendix A.



Existing Conditions

The 5no. development sites are all previously undeveloped and are considered to be greenfield.

Topographic levels to metres Above Ordnance Datum (m AOD) have been derived from a 1m resolution Environment Agency (EA) composite 'Light Detecting and Ranging' (LiDAR) Digital Terrain Model (DTM). A review of LiDAR data shows that the topography of the developable areas of each site vary as follows:

- KS3 Level slope from 36m AOD in the north to 33m AOD in the south.
- KS5 (SHELAA 148b) Levels slope from 41m AOD in the north-west to 37m AOD in the south-west.
- KS7 (SHELAA 80) Levels slope from 37m AOD in the north to 36m AOD in the south-west.
- SHELAA 81 Levels slope from 38m AOD in the south-west to 35m AOD in the north.
- KS6 Levels slope from 37m AOD in the south to 35m AOD in the north.

A LiDAR extract is included in Appendix B. The developable areas of each site are derived from document 'Site Assessment Phase II' provided by the Client.

Ground Conditions

The British Geological Survey (BGS) online mapping (1:50,000 scale) indicates that all the development sites considered in this study are underlain by superficial deposits of Head, generally comprising clay, silt, sand and gravel. The superficial deposits are identified as being underlain by the Newhaven Chalk Formation.

The geological mapping is available at a scale of 1:50,000 and as such may not be accurate on a site-specific basis.

Historical BGS borehole records are available in the western extent of SHELAA 81 (BGS reference: SU33SE9) and immediately south of KS1 (BGS reference: SU33SE41). The borehole records are included in Appendix C.

BGS borehole SU33SE9 in the western extent of SHELAA 81 identifies topsoil to 1.21m below ground level (m.bgl) underlain by ballast (natural superficials) and grit to 3.7m.bgl. The grit is underlain by soft chalk to 30.48m.bgl. The resting groundwater level was identified at 2.43m.bgl.

BGS borehole SU33SE41 south of KS1 identifies soft chalk from ground level to 18.28m.bgl. The resting groundwater level was identified at 4.57m.bgl.

Hydrology

Somborne Stream flows south-west through King's Somborne and generally north of Winchester Road. The location of Somborne Stream is identified on the Location Plan (Appendix A).



Runoff Rate Estimation & Attenuation Storage

The existing greenfield runoff rates for the developable areas of each site have been estimated using the Revitalised Flood Hydrograph Model (ReFH2) method. A summary of the greenfield runoff rates for a range of events is provided as Appendix D. A summary is also provided in Table 1:

Table 1 - Greenfield Runoff Rates

	KS3	KS5 (SHELAA 148b)	KS7 (SHELAA 80)	SHELAA 81	KS6
Storm Event		Rund	off Rate (litres/sec	ond)	
1 in 2	0.19	0.18	0.09	0.09	0.05
1 in 30	0.44	0.42	0.21	0.21	0.12
1 in 100	0.58	0.54	0.28	0.28	0.16

The greenfield runoff rates are considered to be minimal due to the permeable nature of the underlying geology.

In order to reduce the risk of blockage and ensure drainage systems are self-cleansing, a limited discharge rate of up to 2 l/s may need to be applied to the surface water drainage system for each development site.

Attenuation Storage

In order to achieve a discharge rate of 2 l/s, attenuation storage will be required. An attenuation storage estimate for a typical development of up to 15 dwellings is included in Appendix E. The attenuation storage estimate is based on an impermeable drainage area of 1,240m², equating to 40% of a 3,100m² development site area (equivalent to KS5).

An estimated storage volume of 58m³ will be required to accommodate the 1 in 100 year plus 40% Climate Change (CC) event. The storage estimate is based on storage within a tank or pond structure, a design head of 1m and hydro-brake flow control.

A typical cross section through a geo-cellular attenuation storage tank is provided as Appendix F.

Discharge to Somborne Stream

An assessment of the viability of gravity drainage from each development site to Somborne Stream has been undertaken using LiDAR data. A comparison has been made of the bank level at Somborne Stream in its nearest position to each development site with the approximate invert level of an attenuation storage tank / flow control device. The invert level of an attenuation tank / flow control device is assumed at 1.5m below



ground level, accounting for a 0.8m deep tank and minimum 0.7m of ground cover.

An assessment of the viability of gravity drainage is provided in Table 2.

Table 2 – Gravity Drainage Feasibility

Site	Minimum Ground Level in Developable Area (m AOD)	Approximate Drainage System Invert Level at Flow Control Device (m AOD) – 1.5m below ground level.	Bank Level at Somborne Stream (m AOD)	Gravity Drainage Achievable
KS3	33m AOD	31.5m AOD	32m AOD	Gravity discharge may be feasible subject to limited land level raising and utilisation of shallow depth attenuation storage features (0.4m in height).
KS5 (SHELAA 148b)	37m AOD	35.5m AOD	34m AOD	Gravity connection appears feasible subject to route of piped outfall
KS7 (SHELAA 80)	36m AOD	34.5m AOD	34.5m AOD	Gravity connection appears feasible subject to route of piped outfall. Limited ground level raising may be required.
SHELAA 81	35m AOD	33.5m AOD	34m AOD	Gravity discharge may be feasible subject to limited land level raising and utilisation of shallow depth attenuation storage features (0.4m in height).
KS6	35m AOD	33.5m AOD	34m AOD	Gravity discharge may be feasible subject to limited land level raising and utilisation of shallow depth attenuation storage features (0.4m in height).

It can be concluded that a gravity drainage solution is achievable however would be subject to utilising shallow depth attenuation storage (0.4m high geo-cellular systems or box culverts) and limited site level



raising on sites KS3, SHELAA 81 and KS6. Where ground level raising is not permitted (due to other planning constraints) and sufficient space is not made available for shallow depth attenuation storage solutions, a pumped solution may be required.

Future Development Considerations

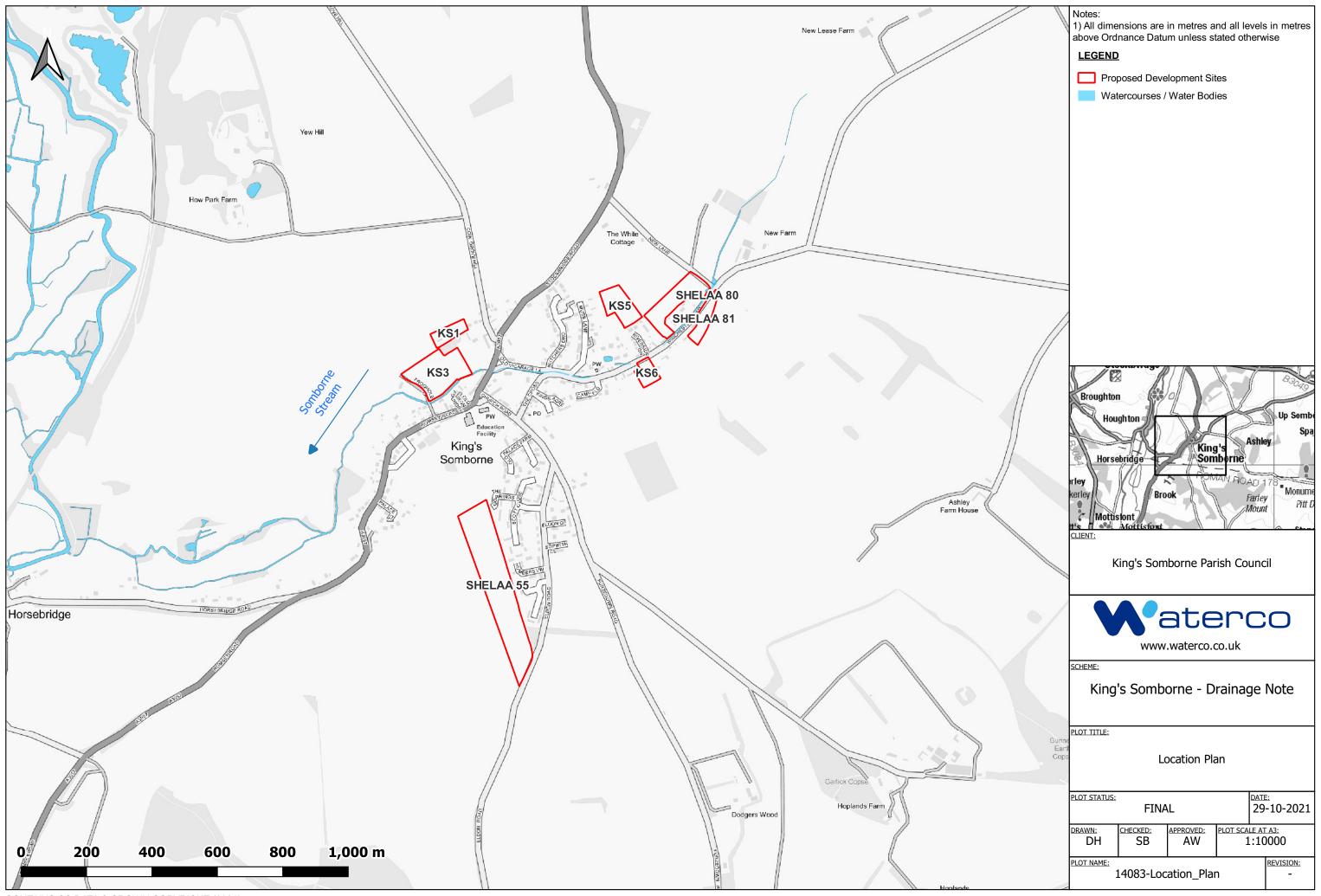
The following information should be provided to support future planning applications / discharge of conditions applications as appropriate:

- Flood Risk Assessment (FRA) for sites within or adjacent to Flood Zone 2 and Flood Zone 3. The FRA
 should consider flood risk from Somborne Stream and should be supported by a detailed hydraulic
 modelling study. Consideration should be given to the flood risk associated with blockage of
 structures (culverts, bridges etc.) along Somborne Stream.
- Ground Investigations including groundwater monitoring (duration of monitoring to be agreed with
 the Lead Local Flood Authority) to confirm the underlying geological sequence and establish seasonal
 groundwater levels. Infiltration tests in accordance with the BRE365 specification should be
 undertaken where infiltration techniques (soakaways and permeable surfaces) can be used on site
 i.e. where the invert level of a soakaway or other infiltration device can be set a minimum of 1m
 above the highest recorded groundwater level.
- Drainage Strategy setting out how surface water and foul flows will be managed. Where surface water discharge to Somborne Stream is made, the Drainage Strategy should detail how a limited discharge rate of 2 I/s (or lower where possible) will be achieved (provide details of flow control and attenuation storage). The Drainage Strategy should also consider the requirement for a non-return valve on the surface water and foul drainage system and should also consider storm water storage requirements in the event that the outfall to Somborne Stream becomes surcharged (submerged in flooding conditions).
- Detailed Drainage Design setting out the drainage layout and levels. Where the invert level of below
 ground attenuation feature is within 1m of the identified groundwater level, groundwater floatation
 calculations should be undertaken, and appropriate mitigation specified where required to prevent
 floatation of the attenuation storage feature.



Appendix A Location Plan and Aerial Image

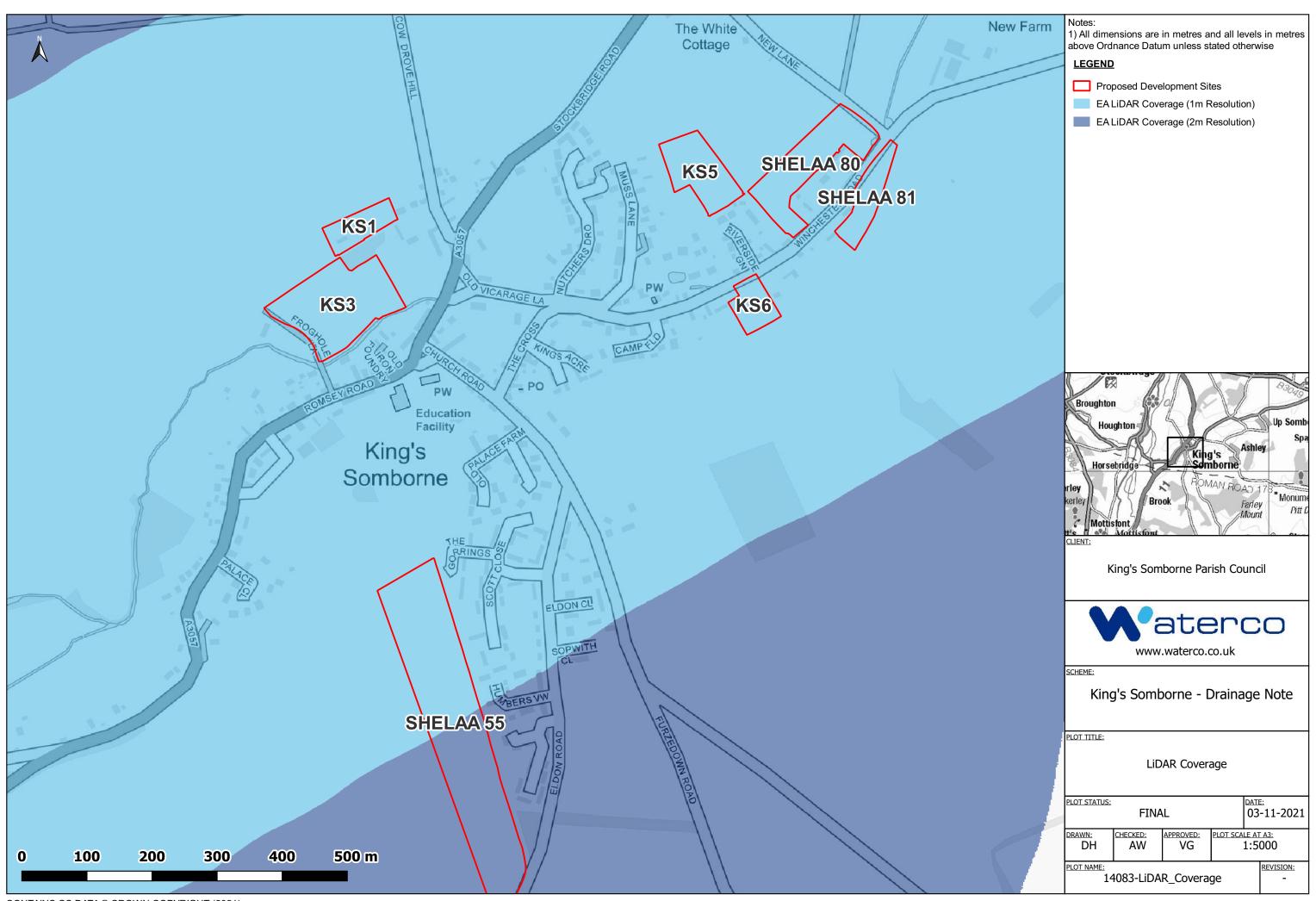


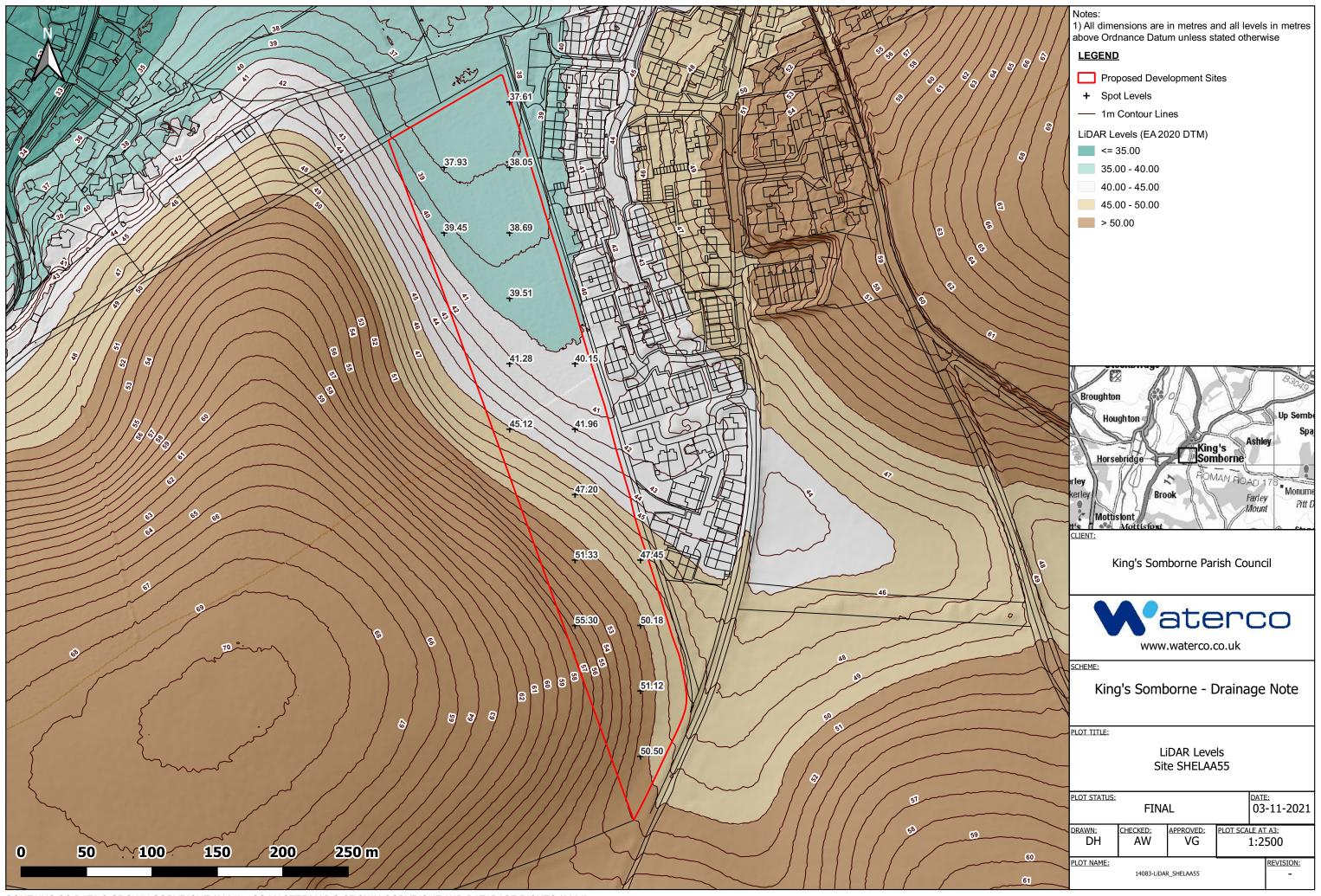


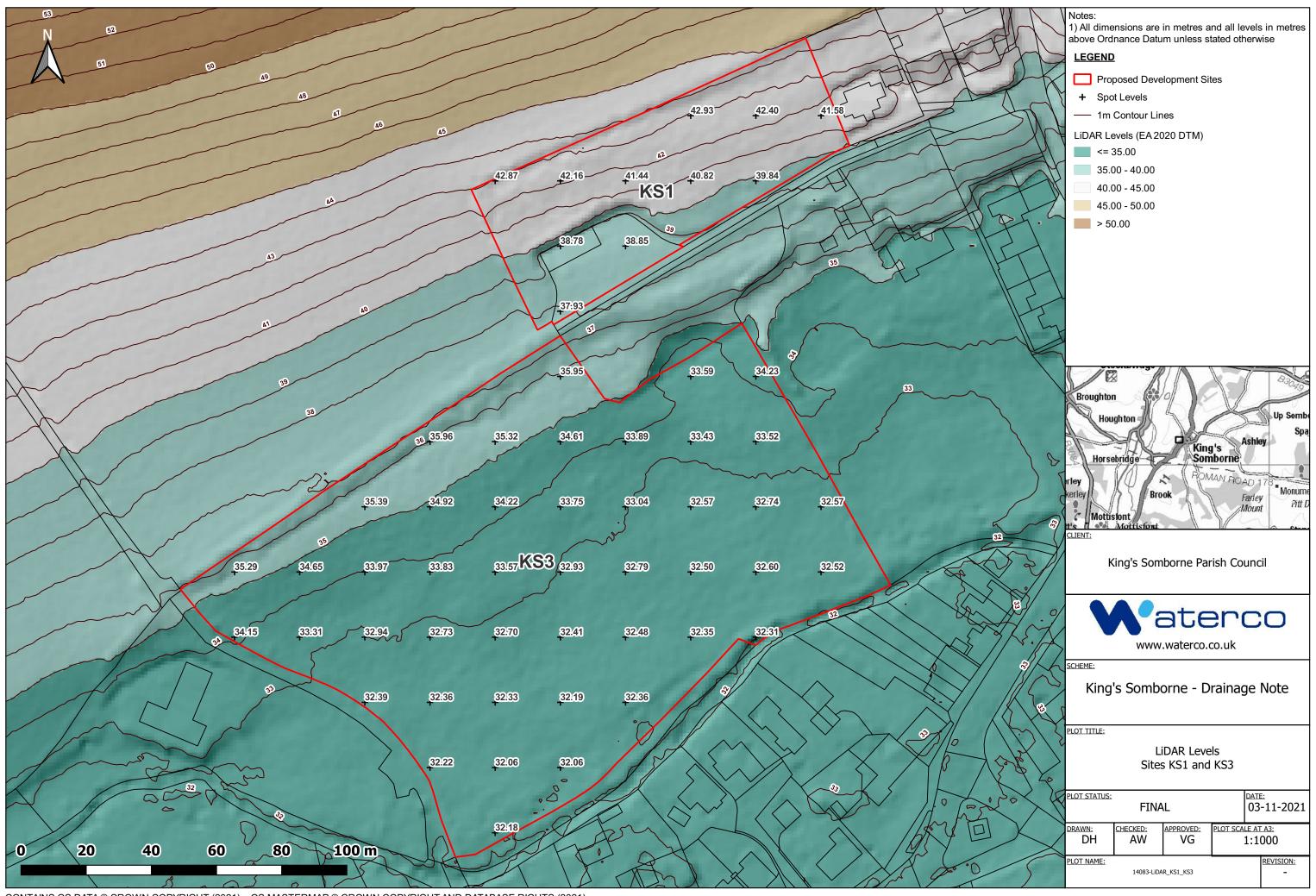


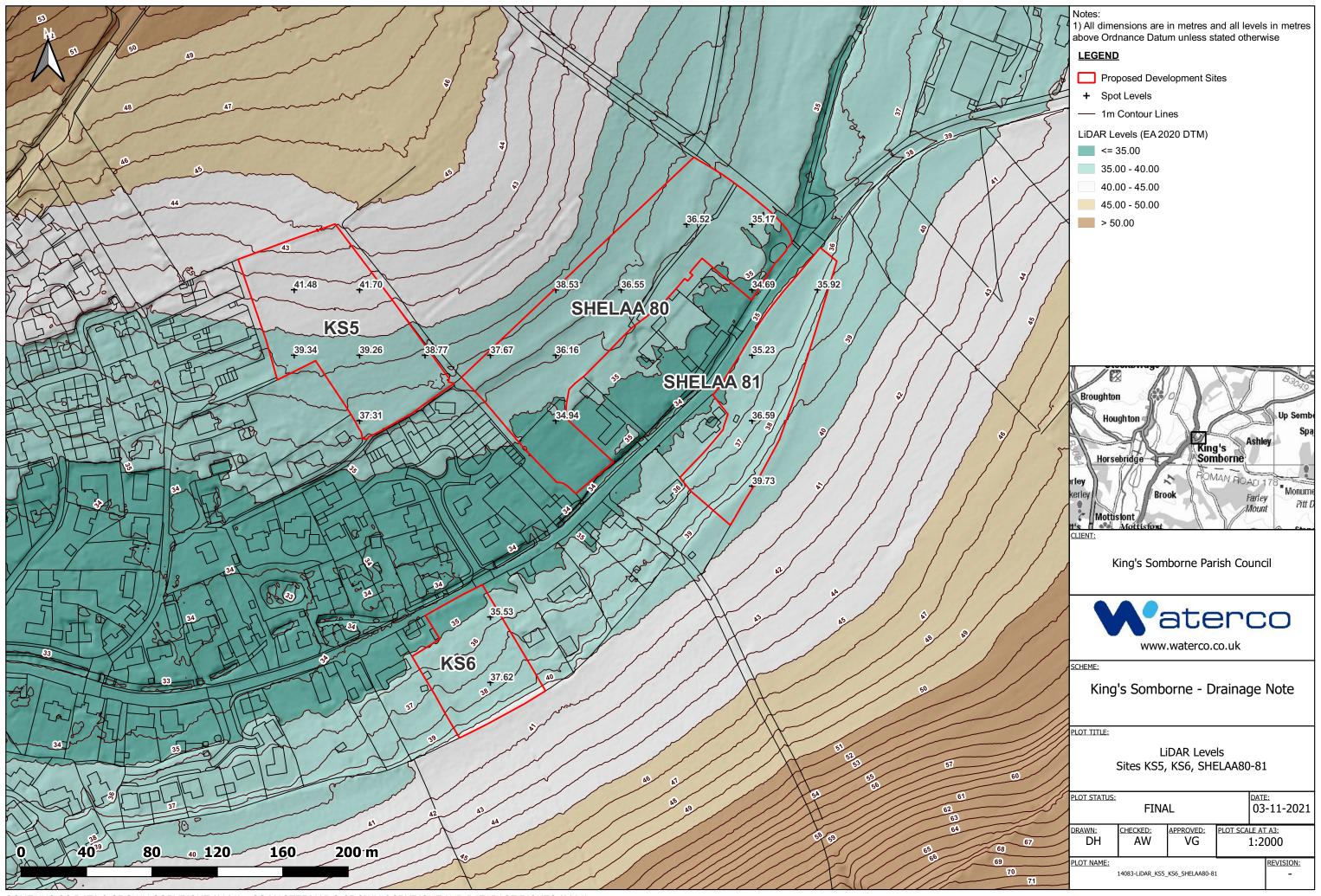
Appendix B LiDAR Extract





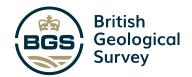






Appendix C BGS Borehole Records





BGS ID: 406969 : BGS Reference: SU33SE9 British National Grid (27700) : 436690,131230

Report an issue with this borehole



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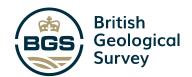
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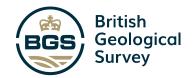


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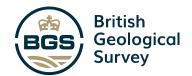
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SOUTH KENSINGTON,
LONDON, S.W.7.

LONDON, S.W.7.

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BGS ID: 407001 : BGS Reference: SU33SE41 British National Grid (27700) : 435960,131210

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Appendix D ReFH2 Greenfield Runoff Rates



King's Somborne ReFH2 Runoff Rates

DOCUMENT VERIFICATION RECORD				
Project:	King's Somborne			
Client:	King's Somborne Parish Council			
Report Title:	14083-Drainage Note-01			
Date:	3 rd November 2021			

DOCUMENT REVIEW & APPROVAL				
Author:	Aled Williams BSc (Hons) MCIWEM			
Checker:	David Hughes Meng (Hons) GMICE			
Approver:	Victoria Griffin BSc (Hons) MSc MIEnvSc CEnv			

ReFH2 RUNOFF I	RATES (I/s) *				
Return Period (Years)	KS3	KS5 (SHELAA 148b)	KS7 (SHELAA 80)	SHELAA 81	KS6
1	0.17	0.16	0.08	0.08	0.046
2	0.19	0.18	0.09	0.09	0.05
5	0.28	0.26	0.13	0.13	0.075
10	0.34	0.32	0.16	0.16	0.09
30	0.44	0.42	0.21	0.21	0.12
50	0.50	0.47	0.24	0.24	0.14
75	0.54	0.51	0.26	0.26	0.15
100	0.58	0.54	0.28	0.28	0.16
200	0.67	0.63	0.33	0.33	0.18
1000	1.02	0.96	0.49	0.49	0.28

^{*}Runoff Rates printed from the ReFH Flood Modelling software package



Appendix E Attenuation Storage Estimate



Waterco Ltd		Page 1
Eden Court	14083 - Kings Somborne	
Lon Parcwr Business Park		
Denbighshire LL15 1NJ	1 in 100 year plus 40% CC	Micro
Date 28/09/2021 10:56	Designed by JW	Designation
File	Checked by AW	Dialilade
XP Solutions	Source Control 2020.1.3	

Summary of Results for 100 year Return Period (+40%)

	Stor Even		Max Level (m)	Max Depth (m)	Max Control (1/s)		Status
15	min	Summer	9.499	0.499	1.9	29.9	ОК
30	min	Summer	9.658	0.658	1.9	39.5	O K
60	min	Summer	9.807	0.807	1.9	48.4	Flood Risk
120	min	Summer	9.841	0.841	1.9	50.4	Flood Risk
180	min	Summer	9.832	0.832	1.9	49.9	Flood Risk
240	min	Summer	9.810	0.810	1.9	48.6	Flood Risk
360	min	Summer	9.771	0.771	1.9	46.2	Flood Risk
480	min	Summer	9.735	0.735	1.9	44.1	Flood Risk
600	min	Summer	9.701	0.701	1.9	42.1	Flood Risk
720	min	Summer	9.668	0.668	1.9	40.1	O K
960	min	Summer	9.602	0.602	1.9	36.1	O K
1440	min	Summer	9.469	0.469	1.9	28.1	O K
2160	min	Summer	9.325	0.325	1.9	19.5	O K
2880	min	Summer	9.229	0.229	1.9	13.8	O K
4320	min	Summer	9.132	0.132	1.8	7.9	O K
5760	min	Summer	9.091	0.091	1.6	5.4	O K
7200	min	Summer	9.076	0.076	1.4	4.6	O K
8640	min	Summer	9.067	0.067	1.3	4.0	O K
10080	min	Summer	9.061	0.061	1.2	3.6	O K
15	min	Winter	9.561	0.561	1.9	33.7	O K
30	min	Winter	9.740	0.740	1.9	44.4	Flood Risk

Storm Event		Rain (mm/hr)		Discharge Volume	Time-Peak (mins)	
				(m³)	(m³)	, ,
15	min	Summer	134.448	0.0	31.2	16
30	min	Summer	90.446	0.0	42.0	31
60	min	Summer	57.968	0.0	53.9	62
120	min	Summer	33.050	0.0	61.4	120
180	min	Summer	23.801	0.0	66.4	180
240	min	Summer	18.869	0.0	70.2	216
360	min	Summer	13.629	0.0	76.0	278
480	min	Summer	10.841	0.0	80.6	342
600	min	Summer	9.089	0.0	84.5	410
720	min	Summer	7.875	0.0	87.9	482
960	min	Summer	6.289	0.0	93.5	624
1440	min	Summer	4.601	0.0	102.7	866
2160	min	Summer	3.378	0.0	113.1	1232
2880	min	Summer	2.723	0.0	121.5	1560
4320	min	Summer	2.029	0.0	135.8	2248
5760	min	Summer	1.662	0.0	148.3	2944
7200	min	Summer	1.435	0.0	160.1	3672
8640	min	Summer	1.280	0.0	171.4	4400
10080	min	Summer	1.169	0.0	182.5	5136
15	min	Winter	134.448	0.0	35.0	16
30	min	Winter	90.446	0.0	47.1	31

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Eden Court	14083 - Kings Somborne	
Lon Parcwr Business Park		
Denbighshire LL15 1NJ	1 in 100 year plus 40% CC	Micro
Date 28/09/2021 10:56	Designed by JW	Designation
File	Checked by AW	Diamade
XP Solutions	Source Control 2020.1.3	

Summary of Results for 100 year Return Period (+40%)

	Stor: Even		Max Level (m)	Max Depth (m)	Max Control (1/s)	Max Volume (m³)	Status
60	min	Winter	9.913	0.913	1.9	54.8	Flood Risk
120	min	Winter	9.960	0.960	2.0	57.6	Flood Risk
180	min	Winter	9.959	0.959	2.0	57.5	Flood Risk
240	min	Winter	9.940	0.940	1.9	56.4	Flood Risk
360	min	Winter	9.888	0.888	1.9	53.3	Flood Risk
480	min	Winter	9.843	0.843	1.9	50.6	Flood Risk
600	min	Winter	9.796	0.796	1.9	47.7	Flood Risk
720	min	Winter	9.749	0.749	1.9	44.9	Flood Risk
960	min	Winter	9.654	0.654	1.9	39.2	O K
1440	min	Winter	9.444	0.444	1.9	26.7	O K
2160	min	Winter	9.244	0.244	1.9	14.6	O K
2880	min	Winter	9.144	0.144	1.8	8.6	O K
4320	min	Winter	9.078	0.078	1.5	4.7	O K
5760	min	Winter	9.062	0.062	1.2	3.7	O K
7200	min	Winter	9.054	0.054	1.1	3.2	O K
8640	min	Winter	9.049	0.049	0.9	2.9	O K
10080	min	Winter	9.046	0.046	0.9	2.7	O K

Storm		Rain	${\tt Flooded}$	Discharge	Time-Peak	
	Even	t	(mm/hr)	Volume	Volume	(mins)
				(m³)	(m³)	
60	min	Winter	57.968	0.0	60.3	60
120	min	Winter	33.050	0.0	68.8	118
180	min	Winter	23.801	0.0	74.3	174
240	min	Winter	18.869	0.0	78.6	228
360	min	Winter	13.629	0.0	85.1	288
480	min	Winter	10.841	0.0	90.3	366
600	min	Winter	9.089	0.0	94.6	444
720	min	Winter	7.875	0.0	98.4	520
960	min	Winter	6.289	0.0	104.8	674
1440	min	Winter	4.601	0.0	115.0	924
2160	min	Winter	3.378	0.0	126.6	1256
2880	min	Winter	2.723	0.0	136.1	1584
4320	min	Winter	2.029	0.0	152.1	2208
5760	min	Winter	1.662	0.0	166.1	2944
7200	min	Winter	1.435	0.0	179.3	3648
8640	min	Winter	1.280	0.0	192.0	4408
10080	min	Winter	1.169	0.0	204.4	5104

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Eden Court	14083 - Kings Somborne	
Lon Parcwr Business Park		
Denbighshire LL15 1NJ	1 in 100 year plus 40% CC	Micco
Date 28/09/2021 10:56	Designed by JW	Designation
File	Checked by AW	Diamarie
XP Solutions	Source Control 2020.1.3	·

Rainfall Details

Rainfall Model						FEH
Return Period (years)						100
FEH Rainfall Version						2013
Site Location	GB	436018	131029	SU	36018	31029
Data Type						Point
Summer Storms						Yes
Winter Storms						Yes
Cv (Summer)						0.750
Cv (Winter)						0.840
Shortest Storm (mins)						15
Longest Storm (mins)						10080
Climate Change %						+40

Time Area Diagram

Total Area (ha) 0.124

 Time From:
 (mins) (ha)

 0
 1

 0
 1

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Eden Court	14083 - Kings Somborne	
Lon Parcwr Business Park		
Denbighshire LL15 1NJ	1 in 100 year plus 40% CC	Micro
Date 28/09/2021 10:56	Designed by JW	Desipago
File	Checked by AW	Diamage
XP Solutions	Source Control 2020.1.3	·

Model Details

Storage is Online Cover Level (m) 10.000

Tank or Pond Structure

Invert Level (m) 9.000

Depth (m) Area (m²) Depth (m) Area (m²) 0.000 60.0 1.000 60.0

Hydro-Brake® Optimum Outflow Control

Unit Reference MD-SHE-0067-2000-1000-2000 Design Head (m) 1.000 Design Flow (1/s) 2.0 Flush-Flo™ Calculated Objective Minimise upstream storage Application Surface Sump Available Diameter (mm) 67 Invert Level (m) 8.995 Minimum Outlet Pipe Diameter (mm) 100 Suggested Manhole Diameter (mm) 1200

Control Points Head (m) Flow (1/s)

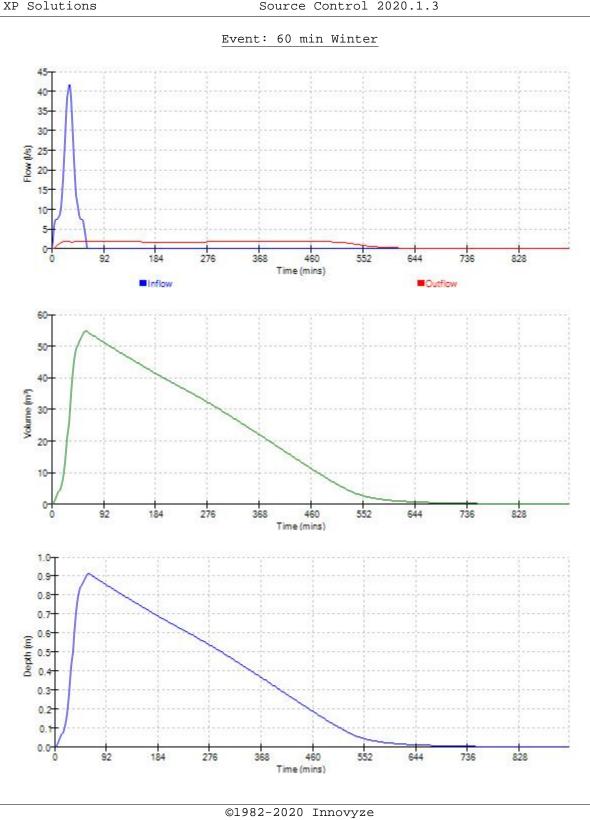
Design	Point	(Calculated)	1.000	2.0
		Flush-Flo™	0.296	1.9
		Kick-Flo®	0.599	1.6
Mean F	low ove	r Head Range	_	1.7

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

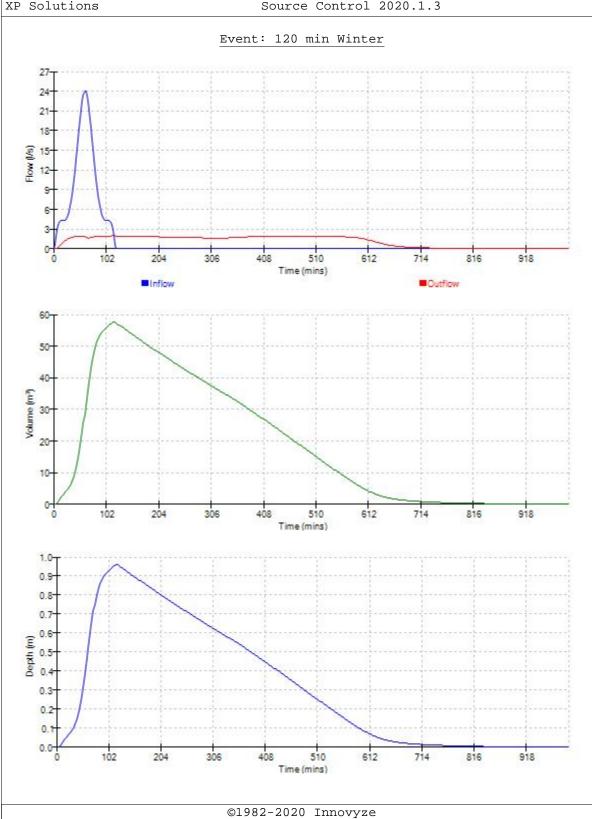
Depth (m) F	low (1/s)	Depth (m) Flo	w (1/s)	Depth (m) Flow	(1/s)	Depth (m)	Flow (1/s)
0.100	1.6	1.200	2.2	3.000	3.3	7.000	4.9
0.200	1.9	1.400	2.3	3.500	3.5	7.500	5.1
0.300	1.9	1.600	2.5	4.000	3.8	8.000	5.2
0.400	1.9	1.800	2.6	4.500	4.0	8.500	5.4
0.500	1.8	2.000	2.7	5.000	4.2	9.000	5.5
0.600	1.6	2.200	2.9	5.500	4.4	9.500	5.7
0.800	1.8	2.400	3.0	6.000	4.6		
1.000	2.0	2.600	3.1	6.500	4.7		

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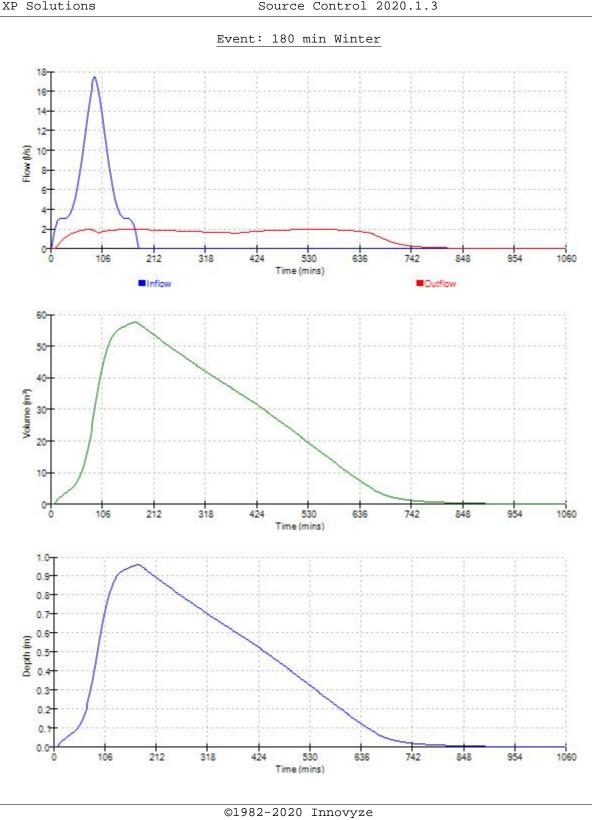
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Lon Parcwr Business Park		
Denbighshire LL15 1NJ	1 in 100 year plus 40% CC	Micco
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XP Solutions	Source Control 2020 1 3	



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Denbighshire LL15 1NJ	1 in 100 year plus 40% CC	Micro
Date 28/09/2021 10:56	Designed by JW	Drainage
File	Checked by AW	Dialilade
XP Solutions	Source Control 2020.1.3	

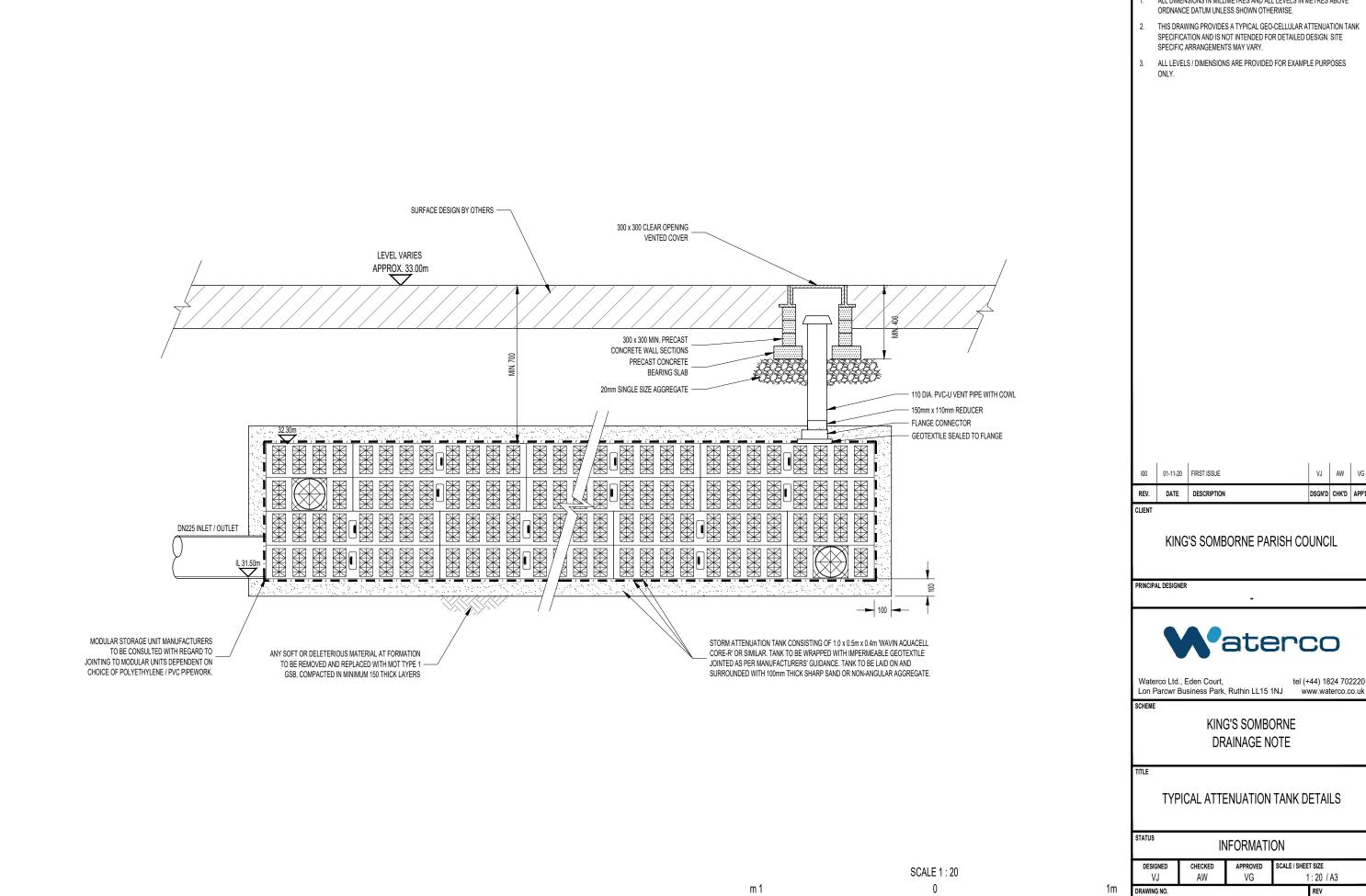


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Denbighshire LL15 1NJ	1 in 100 year plus 40% CC	Micro
Date 28/09/2021 10:56	Designed by JW	Drainage
File	Checked by AW	Diali lack
XP Solutions	Source Control 2020.1.3	<u> </u>



Appendix F Typical Attenuation Tank Cross Section





NOTES

- ALL DIMENSIONS IN MILLIMETRES AND ALL LEVELS IN METRES ABOVE
- THIS DRAWING PROVIDES A TYPICAL GEO-CELLULAR ATTENUATION TANK SPECIFICATION AND IS NOT INTENDED FOR DETAILED DESIGN. SITE

VJ AW VG DSGN'D CHK'D APP'D

KING'S SOMBORNE PARISH COUNCIL



tel (+44) 1824 702220

TYPICAL ATTENUATION TANK DETAILS

APPROVED SCALE / SHEET SIZE 1:20 / A3 100 14083-2401